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Authorised and notified according to Article 29 of the Regulation (EU) No 305/2011 of the European Parliament and of the Council of 9 March 2011

European Technical Assessment ETA-10/0007 of 2015-01-26

I General Part

Technical Assessment Body issuing the ETA and designated according to Article 29 of the Regulation (EU) No 305/2011: ETA-Danmark A/S

Trade name of the construction product:	GH Purlin ties Uni 170, 210 and 250 and right/left 170, 210, 250, 290, 330 and 370
Product family to which the above construction product belongs:	Purlin tie for timber-to-timber connections(timber-to-timber purlin tie)
Manufacturer:	GH-Baubeschläge GmbH Austraße 34 D-73235 Weilheim/Teck Tel. +49 7023 743323 0 Fax +49 7023 743323 90 Internet www.holzverbinder.de
Manufacturing plant:	Werk 1, Werk 2
This European Technical Assessment contains:	14 pages including 2 annexes which form an integral part of the document
This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of:	Guideline for European Technical Approval (ETAG) No. 015 Three Dimensional Nailing Plates, April 2013, used as European Assessment Document (EAD).
This version replaces:	The previous ETA with the same number issued on 2010-01-26 and expiry on 2015-01-26

Translations of this European Technical Assessment in other languages shall fully correspond to the original issued document and should be identified as such.

II SPECIFIC PART OF THE EUROPEAN TECHNICAL ASSESSMENT

1 Technical description of product and intended use

Technical description of the product
GH Purlin ties uni 170, 210 and 250 and right/left 170, 210, 250, 290, 330 and 370 are one-piece non-welded, face-fixed purlin ties to be used in timber to timber connections. They are connected to the timber elements by threaded nails or self-tapping screws.

The purlin ties are either made from pre-galvanized steel S 350 GD + Z 275 according to EN 10346:2009 (only GH Purlin ties uni 170, 210 and 250 made of 1,5 mm steel plates) or S 250 GD + Z 275 according to EN 10346:2009 or DX51D + Z (min Z275) according to EN 10327:2004 or stainless steel 1.4301, 1.4401, 1.4541 or 1.4571 according to EN 10088-2:2005 with a minimum R_e of 250 MPa, a maximum tensile strength R_m of 360 MPa and a minimum ultimate strain A_{80} of 19 % (GH purlin ties made of 2,0 mm steel plates). Dimensions, hole positions and typical installations are shown in Annex A. Purlin ties are made from steel with tolerances according to EN 10143.

2 Specification of the intended use in accordance with the applicable EAD

The purlin ties are intended for use in making connections in load bearing timber structures, as a connection between a beam and a purlin, where requirements for mechanical resistance and stability and safety in use in the sense of the Basic Works Requirements 1 and 4 of Regulation (EU) 305/2011 shall be fulfilled.

The connection always contains two purlin ties (see Annex A).

The static and kinematic behaviour of the timber members or the supports shall be as described in Annex B.
The wood members may be of solid timber, glued laminated timber and similar glued members, or wood-based structural members with a characteristic density from 290 kg/m^3 to 420 kg/m^3. This requirement to the material of the wood members can be fulfilled by using the following materials:

- Structural solid timber classified to C14-C40 according to EN 338 / EN 14081,
- Glulam classified to GL24-GL36 according to EN 1194 / EN 14080,
- LVL according to EN 14374,
- Parallam PSL,
- Intrallam LSL,
- Duo- and Triobalken,
- Layered wood plates,
- Plywood according to EN 636

Annex B states the load-carrying capacities of the purlin tie connections for a characteristic density of 350 kg/m^3. For timber or wood based material with a different characteristic density than 350 kg/m^3 the load-carrying capacities of the nailed connection shall be modified by the k_{dens} factor:

$$k_{dens} = \sqrt{\frac{\rho_k}{350}}$$

Where ρ_k is he characteristic density of the timber in kg/m^3.

The design of the connections shall be in accordance with Eurocode 5 or a similar national Timber Code. The wood members shall have a thickness which is larger than the penetration depth of the nails into the members.

The zinc-coated purlin ties are primarily for use in timber structures subject to the dry, internal conditions defined by service class 1 and 2 of Eurocode 5 and for connections subject to static or quasi-static loading.

The purlin ties can also be used in outdoor timber structures, service class 3, when a corrosion protection in accordance with Eurocode 5 is applied, or when stainless steel with similar or better characteristic yield and ultimate strength is employed.

The scope of the purlin ties regarding resistance to corrosion shall be defined according to national provisions that apply at the installation site considering environmental conditions.

The provisions made in this European Technical Assessment are based on an assumed intended working life of the connectors of 50 years.

The indications given on the working life cannot be interpreted as a guarantee given by the producer or Assessment Body, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

3 Performance of the product and references to the methods used for its assessment

Characteristic	Assessment of characteristic
3.1 Mechanical resistance and stability*) (BWR1)	
Characteristic load-carrying capacity	See Annex B
Stiffness	No performance determined
Ductility in cyclic testing	No performance determined
3.2 Safety in case of fire (BWR2)	
Reaction to fire	The purlin ties are made from steel classified as **Euroclass A1** in accordance with EN 1350-1 and EC decision 96/603/EC, amended by EC Decision 2000/605/EC
3.3 Hygiene, health and the environment (BWR3)	
Influence on air quality	The product does not contain/release dangerous substances specified in TR 034, dated March 2012 0**)
3.7 Sustainable use of natural resources (BWR7)	No Performance Determined
3.8 General aspects related to the performance of the product	The purlin ties have been assessed as having satisfactory durability and serviceability when used in timber structures using the timber species described in Eurocode 5 and subject to the conditions defined by service class 1 and 2

*) See additional information in section 3.8 – 3.9.

**) In addition to the specific clauses relating to dangerous substances contained in this European technical Assessment, there may be other requirements applicable to the products falling within its scope (e.g. transposed European legislation and national laws, regulations and administrative provisions). In order to meet the provisions of the Construction Products Regulation, these requirements need also to be complied with, when and where they apply.

3.9 Methods of verification

The characteristic load-carrying capacities are based on the characteristic values of the nail or screw connections, the timber components and the steel plates. To obtain design values the capacities have to be multiplied with different partial factors for the material properties, the nailed or screwed connection and the timber components in addition with the coefficient k_{mod}.

According to EN 1990 (Eurocode – Basis of design) paragraph 6.3.5 the design value of load-carrying capacity can be determined by reducing the characteristic values of the load-carrying capacity with different partial factors.

Thus, the characteristic values of the load–carrying capacity are determined also for timber failure $F_{Rk,H}$ (reaching the embedment strength of nails or screws subjected to shear), $F_{90,Rk}$ (reaching the transverse tensile strength of the timber components) as well as for steel plate failure $F_{Rk,S}$. The design value of the load–carrying capacity is the smaller value of both load–carrying capacities.

$$F_{Rd} = \min\left\{\frac{k_{mod} \cdot F_{Rk,N}}{\gamma_{M,H}}; \frac{F_{Rk,S}}{\gamma_{M,S}}; \frac{k_{mod} \cdot F_{90,Rk}}{\gamma_{M,H}}\right\}$$

Therefore, for timber failure and the nailed or screwed connection the load duration class and the service class are included. The different partial factors γ_M for steel or timber, respectively, are also correctly taken into account.

3.10 Mechanical resistance and stability

See annex B for the characteristic load-carrying capacity in the direction F_1.

The characteristic capacities of the purlin ties are determined by calculation assisted by testing as described in the EOTA Guideline 015 clause 5.1.2. They should be used for designs in accordance with Eurocode 5 or a similar national Timber Code.

Threaded nails (ringed shank nails) in accordance to EN 14592
In the formulas in Annex B the capacities for threaded nails calculated from the formulas of Eurocode 5 are used assuming a thick steel plate when calculating the lateral nail load-carrying-capacity.

The load bearing capacities of the brackets has been determined based on the use of connector nails 4,0 x 40 mm in accordance with the German national approval for the nails.

The characteristic withdrawal capacity of the nails has to be determined by calculation in accordance with EN 1995-1-1: 2004, paragraph 8.3.2 (head pull-through is not relevant):

$$F_{ax,Rk} = f_{ax,k} \times d \times t_{pen}$$

Where:

$f_{ax,k}$ Characteristic value of the withdrawal parameter in N/mm^2
d Nail diameter in mm
t_{pen} Penetration depth of the profiles in mm

Based on tests by Versuchsanstalt für Stahl, Holz und Steine, University of Kalrsruhe, the characteristic value of the withdrawal resistance for the threaded nails used can be calculated as:

$$f_{ax,k} = 50 \times 10^{-6} \times \sigma_k^2$$

Where:

σ_k Characteristic density of the timber in kg/m^3

The shape of the nail directly under the head shall be in the form of a truncated cone with a diameter under the nail head which exceeds the hole diameter.

No performance has been determined in relation to ductility of a joint under cyclic testing. The contribution to the performance of structures in seismic zones, therefore, has not been assessed.

No performance has been determined in relation to the joint's stiffness properties - to be used for the analysis of the serviceability limit state

3.11 Aspects related to the performance of the product

2.7.1 Corrosion protection in service class 1 and 2.
In accordance with ETAG 015 the purlin ties are made from pre-galvanized steel DX 51 D / Z 275 according to EN 10346:2009.

2.7.2 Corrosion protection in service class 3
In accordance with Eurocode 5 the purlin ties with a thickness up to 3 mm shall be made from stainless steel. Purlin ties with a thickness from 3 to 5 mm can

be made from stainless steel or have a zinc coating of min. Fe/Zn 25c/Z350 according to ISO 2081/EN 10147. The nails or screws shall be produced from stainless steel or have a zinc coating of min. Fe/Zn 25c.

3.12 General aspects related to the fitness for use of the product

The performance given in this ETA are based on the following:

- The structural members to which the purlin ties are fixed shall be:
  - Restrained against rotation.
  - Solid timber according to EN 338 or better, see section 3 of this evaluation report
  - Free from wane under the purlin tie.

- The tensile perpendicular to the grain capacity of the timber member to be used in conjunction with the purlin tie is to be checked by the designer of the structure to ensure it is not less than the purlin tie capacity and, if necessary, the purlin tie capacity reduced accordingly.

- The gap between the timber members does not exceed 3 mm.

- There are no specific requirements relating to preparation of the timber members. There are no specific requirements relating to preparation of the timber members.

4 Attestation and verification of constancy of performance (AVCP)

4.1 AVCP system

According to the decision 97/638/EC of the European Commission1, as amended, the system(s) of assessment and verification of constancy of performance (see Annex V to Regulation (EU) No 305/2011) is 2+.

5 Technical details necessary for the implementation of the AVCP system, as foreseen in the applicable EAD

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited at ETA-Danmark

Issued in Copenhagen on 2015-01-26 by

Thomas Bruun
Managing Director, ETA-Danmark

Annex A
Product details

Table A.1 Materials specification

Purlin Ties Type	Thickness (mm)	Steel specification	Coating specification
uni (170-250)	1,5	S350 GD	Z275
uni (170-250)	2,0	S250 GD or DX51D or stainless steel 1.4301, 1.4401, 1.4541 or 1.4571	Z275 for S250 GD or DX51D
right/left (170-370)	2,0	S250 GD or DX51D or stainless steel 1.4301, 1.4401, 1.4541 or 1.4571	Z275 for S250 GD or DX51D

Table A.2 Dimensions

Purlin Ties Type	Length (mm)		Width (mm)	
	min	**max**	**min**	**max**
uni 170	169	172	35,5	36,5
uni 210	209	212	35,5	36,5
uni 250	249	252	35,5	36,5
right/left 170	169	172	34,0	35,0
right/left 210	209	212	34,0	35,0
right/left 250	249	252	34,0	35,0
right/left 290	289	292	34,0	35,0
right/left 330	329	332	34,0	35,0
right/left 370	369	372	34,0	35,0

Table A.3 Fastener specification

FASTENER	Length	Nail type
Nail 4.0 mm	40 mm	Ringed shank nails according to EN 14592
GH screw 5.0 mm	40 mm	Self-tapping screws according to EN 14592
GH-Nail 4.0 mm	40 – 100 mm	Ringed shank nails according to ETA-13/0523
GH-Screw 5.0 mm	35 – 70 mm	Self-tapping screws according to ETA-13/0523

In the load-carrying-capacities of the nailed or screwed connection in Annex B the capacities calculated from the formulas of Eurocode 5 are used assuming a thick steel plate when calculating the lateral fastener load-carrying-capacity. The load-carrying-capacities of the hold downs have been determined based on the use of connector nails ø 4,0 mm or screws ø 5,0 mm in accordance with the German national approval for the nails or the screws. The characteristic withdrawal capacity of the nails or screws has to be determined by calculation in accordance with EN 1995-1-1:2008, paragraph 8.3.2 (head pull-through is not relevant):

$F_{ax,Rk} = f_{1,k} \times d \times t_{pen}$

Where:

$f_{1,k}$ Characteristic value of the withdrawal parameter in N/mm^2

d Nail or screw diameter in mm

t_{pen} Penetration depth of the profiled shank in mm;
(4,0 x 50 mm $t_{pen} \geq 40$ mm; 4,0 x 60 mm $t_{pen} \geq 50$ mm)

Based on tests by Versuchsanstalt für Stahl, Holz und Steine, University of Karlsruhe, the characteristic value of the withdrawal resistance for the threaded nails or screws used can be calculated as:

$f_{1,k} = 50 \times 10^{-6} \times \rho_k^2$ for nails

$f_{1,k} = 80 \times 10^{-6} \times \rho_k^2$ for screws

Where:

ρ_k Characteristic density of the timber in kg/m^3

The shape of the nail or screw directly under the head shall be in the form of a truncated cone with a diameter under the head which fits or exceeds the hole diameter.

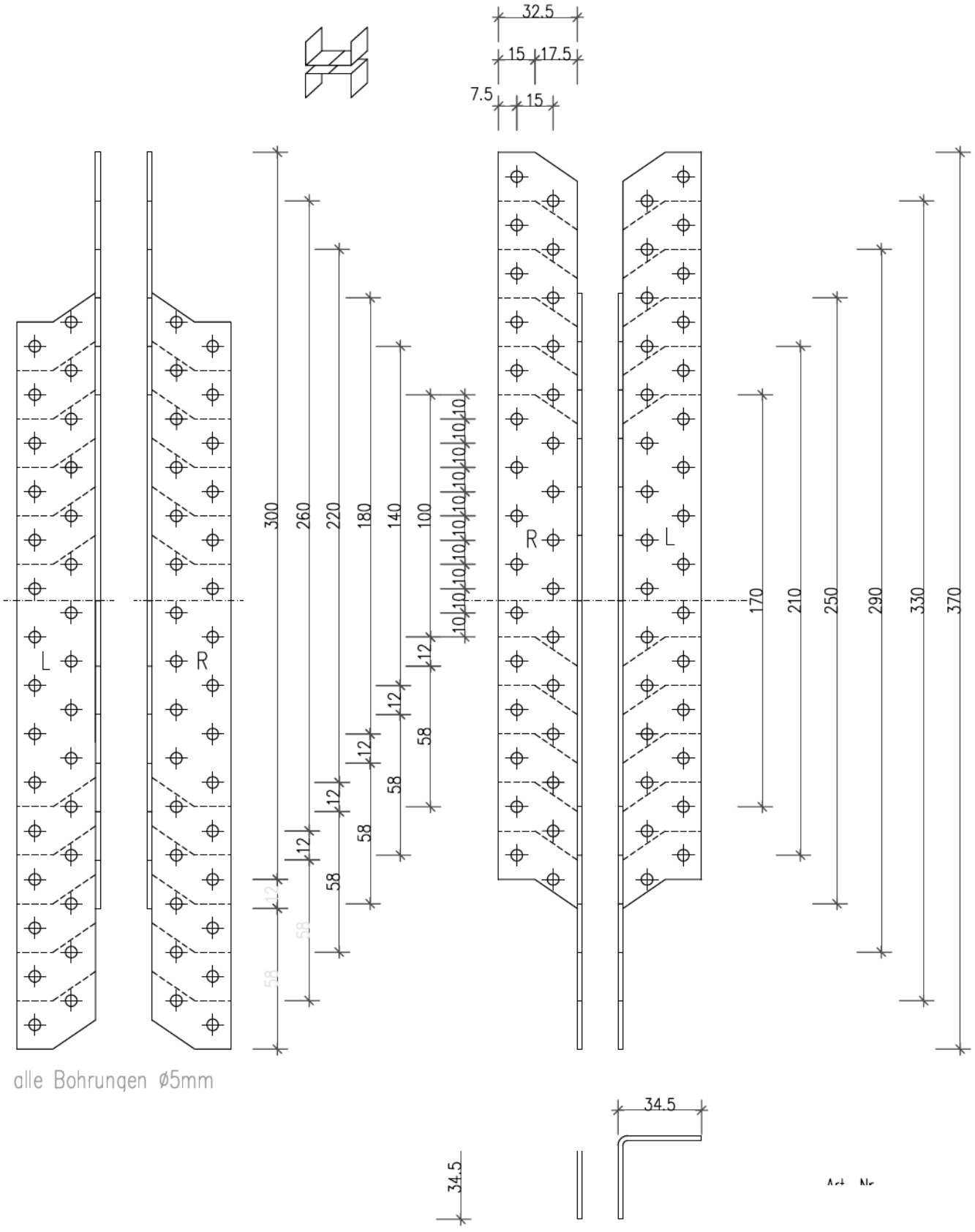

Figure A.1 Dimensions of purlin ties right/left

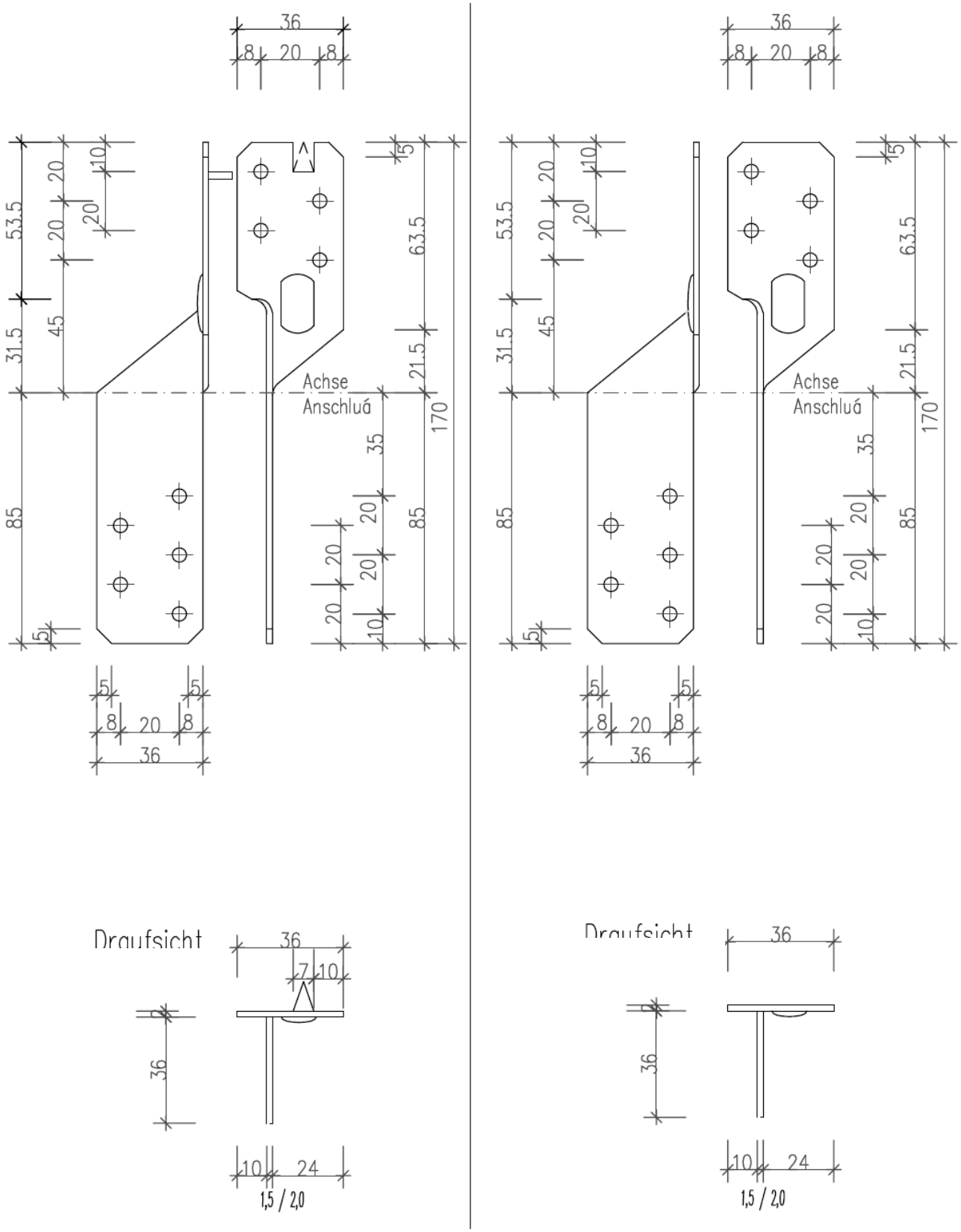

Figure A.2 Dimensions of purlin ties uni 170

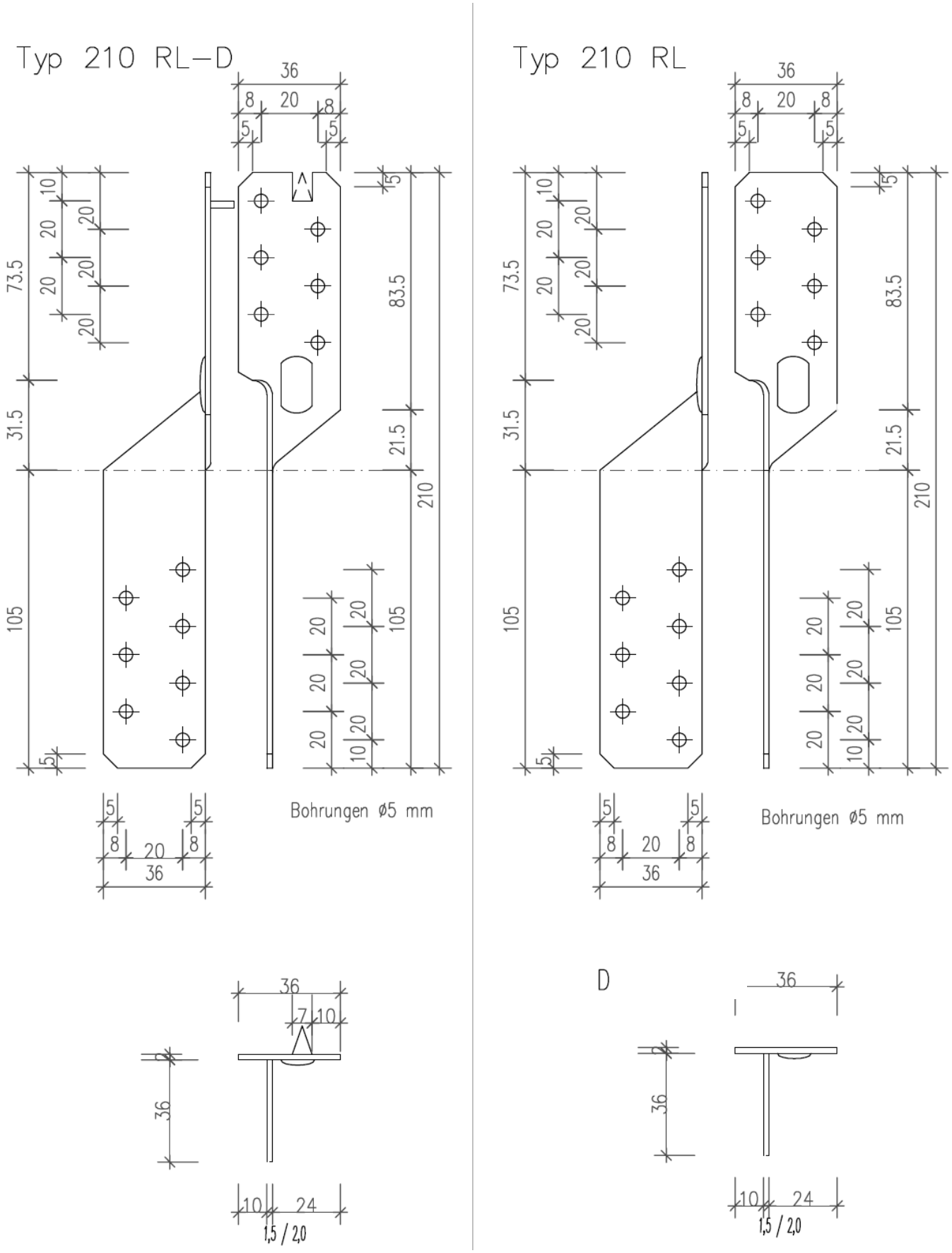

Figure A.3 Dimensions of purlin ties uni 210

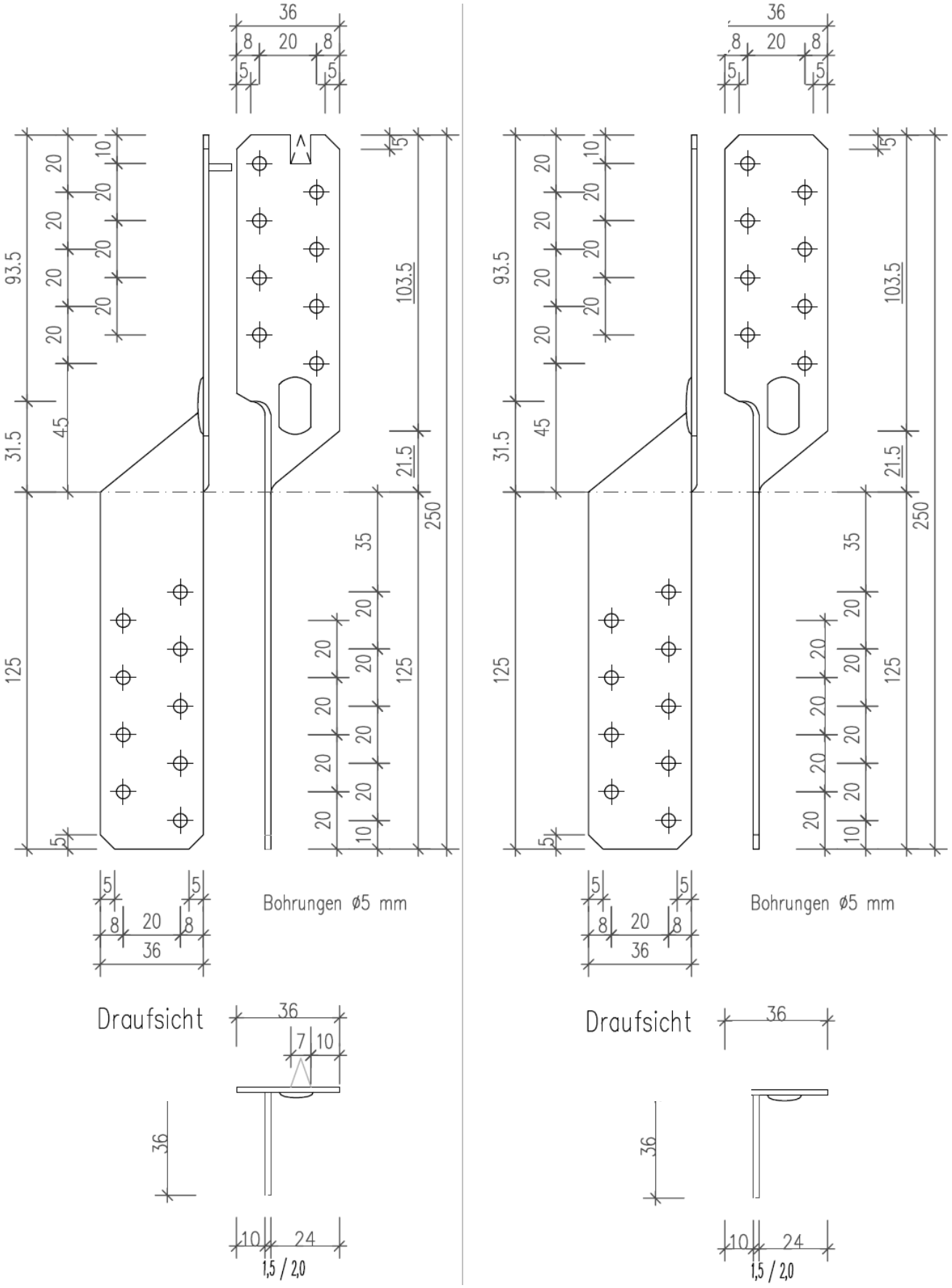

Figure A.4 Dimensions of purlin ties uni 250

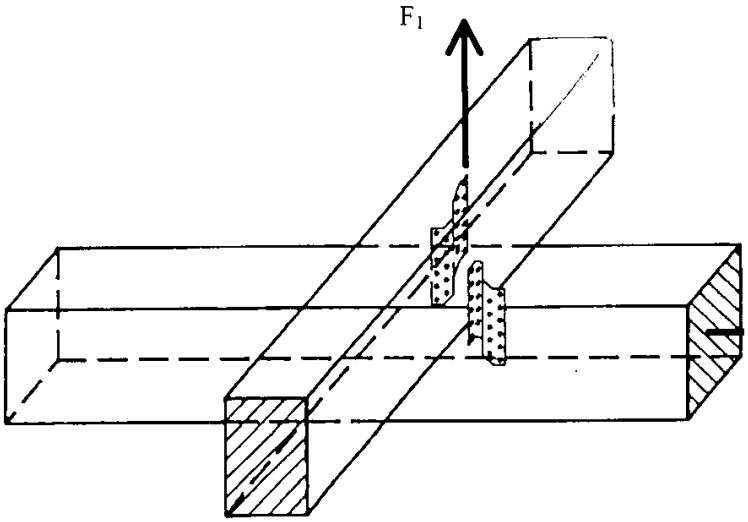

Figure A.5 Typical installation

Annex B
Characteristic load-carrying capacities

Support conditions
The distance between the timber elements in the area of the connection must not exceed 3 mm. The timber members are prevented from rotation.

Fastener specification
The holes have to be nailed or screwed beginning at the end of the purlin tie.

Wane
Wane is not allowed, the timber has to be sharp-edged in the area of the purlin ties.

Characteristic load-carrying capacities 2 purlin ties

Table B.1: Characteristic load-carrying capacities Load F_1 – 2 Purlin Ties / connection

Purlin ties	Number of fasteners per purlin tie	Nailed connection $F_{Rk,N}$	Screwed connection $F_{Rk,N}$	Steel $F_{Rk,S}$	Transverse tensile failure
uni 170 - 250 / 1,5 mm	4 + 5	8,44 kN	10,6 kN	8,1 kN	Design according to equation (B.1)
uni 210 - 250 / 1,5 mm	6 + 7	13,1 kN	16,5 kN	8,1 kN	
uni 250 / 1,5 mm	8 + 9	18,6 kN	23,5 kN	8,1 kN	
uni 170 - 250 / 2,0 mm	4 + 5	8,44 kN	10,6 kN	7,7 kN	
uni 210 - 250 / 2,0 mm	6 + 7	13,1 kN	16,5 kN	7,7 kN	
uni 250 / 2,0 mm	8 + 9	18,6 kN	23,5 kN	7,7 kN	
right/left 170 - 370	2 x 4	5,57 kN	7,01 kN	10,2 kN	
right/left 210 - 370	2 x 6	10,2 kN	12,8 kN	10,2 kN	
right/left 250 - 370	2 x 8	15,7 kN	19,7 kN	10,2 kN	
right/left 290 - 370	2 x 10	21,9 kN	27,6 kN	10,2 kN	
right/left 330 - 370	2 x 12	28,4 kN	35,8 kN	10,2 kN	
right/left 370	2 x 14	35,2 kN	44,4 kN	10,2 kN	

Splitting
For a lifting force F_1 splitting has to be proved, when necessary, for both timber elements. The capacity of a connection with two purlin ties on both sides of the timber element is calculated according to the general splitting design for connections with mechanical fasteners in EN 1995-1-1.

$$F_{90,Rk} = 14 \cdot b \sqrt{\frac{h_e}{\left(1 - \frac{h_e}{h}\right)}} \qquad \text{(B.1)}$$

Where:

$F_{90,Rk}$ the characteristic splitting capacity in N
b the member thickness, in mm
h_e is the loaded edge distance to the centre of the most distant fastener in mm
h the timber member height in mm

The design value of the force component perpendicular to the structural member's axis has to be lower than the design capacity $F_{90,Rd}$.